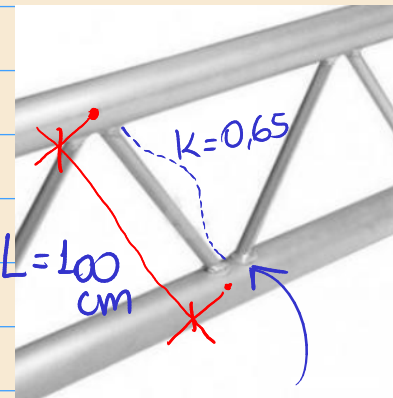
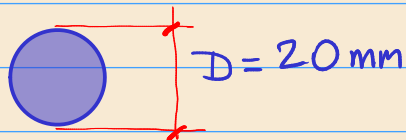


# ROUND SECTION - DETERMINE THE DESIGN COMPRESSIVE STRENGTH.

CONSIDER A CIRCULAR CROSS-SECTION BAR WITH 20 mm OF DIAMETER AND LENGTH ( $L_c$ ) OF 100 cm.



K - EFFECTIVE LENGTH FACTOR

$K = 0.5 \rightarrow$  FOR DESIGN  
WE'LL ADOPT  $K = 0.65$

THEORETICAL VALUE.

WELDED  
ALL THE ROUND

THE SLENDERNESS RATIO

$$\lambda = \frac{L_c}{r} \leq 200$$

EFFECTIVE LENGTH  
RADIUS OF GYRATION


$$r = \frac{D}{4} = \frac{20}{4} = 5 \text{ mm} = 0.5 \text{ cm}$$

You can analyze the effective length or adopt a theoretical length factor.

$$\lambda = \frac{0.65 \cdot 100}{0.5} = 130 < 200 \quad \text{OK!}$$

( lambda

Following THE ROAD MAP

TABLE USER NOTE E1.1 Selection Table for the Application of Chapter E Sections				
Cross Section	Without Slender Elements		With Slender Elements	
	Sections in Chapter E	Limit States	Sections in Chapter E	Limit States
	E3	FB	NA	NA

CHAPTER E3 is APPLICABLE  
FLEXURAL BUCKLING.

We don't have slender elements and as a consequence we don't have local buckling in this case.

### ELASTIC BUCKLING STRESS

$$F_e = \frac{\pi^2 \cdot E}{\left(\frac{L_c}{r}\right)^2} = \frac{\pi^2 \cdot 20000}{130^2} = 11.68 \text{ kN/cm}^2$$

↙ E
↙ kN/cm<sup>2</sup>

Yield Stress

$$f_y = 25 \text{ kN/cm}^2$$

### E3 - FLEXURAL BUCKLING OF MEMBER WITHOUT SLENDER ELEMENTS.

$$4.71 \sqrt{\frac{20000}{25}} = 133.22$$

$$\frac{L_c}{r} \leq 4.71 \sqrt{\frac{E}{f_y}} \Rightarrow F_n = \left( 0.658 \frac{F_y}{F_e} \right) \cdot f_y$$

OR

$$\frac{L_c}{r} > 4.71 \sqrt{\frac{E}{f_y}} \Rightarrow F_n = 0.877 \cdot F_e$$

$$P_n = F_n \cdot A_g \quad \text{GROSS AREA}$$
$$A_g = \frac{\pi \cdot D^2}{4} = \frac{\pi \cdot 2^2}{4} = \underline{\underline{3.1416 \text{ cm}^2}}$$

$$F_n = \left( 0.658 \frac{F_y}{F_e} \right) \cdot F_y$$

$$F_n = \left( 0.658 \frac{25}{11.68} \right) \cdot 25 = 10.206 \text{ kN/cm}^2$$

$$P_n = F_n \cdot A_g = 32,063 \text{ kN}$$

$$\phi_c P_n = 0.9 \times 32,063 = 28,857 \text{ kN}$$